



Given: Cantilevered beam with dimensions shown; rigidly fixed at $x = 0$; applied traction at $x = 18$. $E = 27,000$ ksi; $\nu = 0.25$. Additional loading due to self-weight of steel = 0.003 k/in³.

Required: Using CST plane stress elements, find the approximate deflection of the free end at $y = 0$.

E.B.
 CE 595
 Homework No. 6
 Solution

ORIGIN \equiv 1

Formulate shape functions:

$$\text{ustar}_i(x, y, a_1, a_2, a_3) := a_1 + a_2 \cdot x + a_3 \cdot y$$

$$\text{vstar}_i(x, y, a_4, a_5, a_6) := a_4 + a_5 \cdot x + a_6 \cdot y$$

$$\text{Nbar}_i(x, y) := \begin{pmatrix} 1 & x & y & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x & y \end{pmatrix}$$

Element 1:

$$x1_1 := 0 \quad y1_1 := -1 \quad x1_2 := 6 \quad y1_2 := -1 \quad x1_3 := 0 \quad y1_3 := 1 \quad \text{inches}$$

$$A1 := \begin{pmatrix} 1 & x1_1 & y1_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x1_1 & y1_1 \\ 1 & x1_2 & y1_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x1_2 & y1_2 \\ 1 & x1_3 & y1_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x1_3 & y1_3 \end{pmatrix}$$

$$N1(x, y) := \text{Nbar}_i(x, y) \cdot A1^{-1}$$

$$N1(x, y) \rightarrow \begin{pmatrix} \frac{1}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} & 0 & \frac{y}{2} + \frac{1}{2} & 0 \\ 0 & \frac{1}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} & 0 & \frac{y}{2} + \frac{1}{2} \end{pmatrix}$$

$$N1_1(x, y) := N1(x, y)_{1,1} \quad N1_2(x, y) := N1(x, y)_{1,3} \quad N1_3(x, y) := N1(x, y)_{1,5}$$

Element 2:

$$x2_1 := 6 \quad y2_1 := 1 \quad x2_2 := 0 \quad y2_2 := 1 \quad x2_3 := 6 \quad y2_3 := -1 \quad \text{inches}$$

$$A2 := \begin{pmatrix} 1 & x2_1 & y2_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x2_1 & y2_1 \\ 1 & x2_2 & y2_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x2_2 & y2_2 \\ 1 & x2_3 & y2_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x2_3 & y2_3 \end{pmatrix}$$

$$N2(x,y) := \text{Nbar}_i(x,y) \cdot A2^{-1}$$

$$N2(x,y) \rightarrow \begin{pmatrix} \frac{x}{6} + \frac{y}{2} - \frac{1}{2} & 0 & 1 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} & 0 \\ 0 & \frac{x}{6} + \frac{y}{2} - \frac{1}{2} & 0 & 1 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} \end{pmatrix}$$

$$N2_1(x,y) := N2(x,y)_{1,1} \quad N2_2(x,y) := N2(x,y)_{1,3} \quad N2_3(x,y) := N2(x,y)_{1,5}$$

Element 3:

$$x3_1 := 6 \quad y3_1 := -1 \quad x3_2 := 12 \quad y3_2 := -1 \quad x3_3 := 6 \quad y3_3 := 1 \quad \text{inches}$$

$$A3 := \begin{pmatrix} 1 & x3_1 & y3_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x3_1 & y3_1 \\ 1 & x3_2 & y3_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x3_2 & y3_2 \\ 1 & x3_3 & y3_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x3_3 & y3_3 \end{pmatrix}$$

$$N3(x,y) := \text{Nbar}_i(x,y) \cdot A3^{-1}$$

$$N3(x,y) \rightarrow \begin{pmatrix} \frac{3}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} - 1 & 0 & \frac{y}{2} + \frac{1}{2} & 0 \\ 0 & \frac{3}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} - 1 & 0 & \frac{y}{2} + \frac{1}{2} \end{pmatrix}$$

$$N3_1(x,y) := N3(x,y)_{1,1} \quad N3_2(x,y) := N3(x,y)_{1,3} \quad N3_3(x,y) := N3(x,y)_{1,5}$$

Element 4:

$$x4_1 := 12 \quad y4_1 := 1 \quad x4_2 := 6 \quad y4_2 := 1 \quad x4_3 := 12 \quad y4_3 := -1 \quad \text{inches}$$

$$A4 := \begin{pmatrix} 1 & x4_1 & y4_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x4_1 & y4_1 \\ 1 & x4_2 & y4_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x4_2 & y4_2 \\ 1 & x4_3 & y4_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x4_3 & y4_3 \end{pmatrix}$$

$$N4(x,y) := Nbar_1(x,y) \cdot A4^{-1}$$

$$N4(x,y) \rightarrow \begin{pmatrix} \frac{x}{6} + \frac{y}{2} - \frac{3}{2} & 0 & 2 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} & 0 \\ 0 & \frac{x}{6} + \frac{y}{2} - \frac{3}{2} & 0 & 2 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} \end{pmatrix}$$

$$N4_1(x,y) := N4(x,y)_{1,1} \quad N4_2(x,y) := N4(x,y)_{1,3} \quad N4_3(x,y) := N4(x,y)_{1,5}$$

Element 5:

$$x5_1 := 12 \quad y5_1 := -1 \quad x5_2 := 18 \quad y5_2 := -1 \quad x5_3 := 12 \quad y5_3 := 1 \quad \text{inches}$$

$$A5 := \begin{pmatrix} 1 & x5_1 & y5_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x5_1 & y5_1 \\ 1 & x5_2 & y5_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x5_2 & y5_2 \\ 1 & x5_3 & y5_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x5_3 & y5_3 \end{pmatrix}$$

$$N5(x,y) := \text{Nbar}_1(x,y) \cdot A5^{-1}$$

$$N5(x,y) \rightarrow \begin{pmatrix} \frac{5}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} - 2 & 0 & \frac{y}{2} + \frac{1}{2} & 0 \\ 0 & \frac{5}{2} - \frac{y}{2} - \frac{x}{6} & 0 & \frac{x}{6} - 2 & 0 & \frac{y}{2} + \frac{1}{2} \end{pmatrix}$$

$$N5_1(x,y) := N5(x,y)_{1,1} \quad N5_2(x,y) := N5(x,y)_{1,3} \quad N5_3(x,y) := N5(x,y)_{1,5}$$

Element 6:

$$x6_1 := 18 \quad y6_1 := 1 \quad x6_2 := 12 \quad y6_2 := 1 \quad x6_3 := 18 \quad y6_3 := -1 \quad \text{inches}$$

$$A6 := \begin{pmatrix} 1 & x6_1 & y6_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x6_1 & y6_1 \\ 1 & x6_2 & y6_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x6_2 & y6_2 \\ 1 & x6_3 & y6_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x6_3 & y6_3 \end{pmatrix}$$

$$N6(x,y) := \text{Nbar}_1(x,y) \cdot A6^{-1}$$

$$N6(x,y) \rightarrow \begin{pmatrix} \frac{x}{6} + \frac{y}{2} - \frac{5}{2} & 0 & 3 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} & 0 \\ 0 & \frac{x}{6} + \frac{y}{2} - \frac{5}{2} & 0 & 3 - \frac{x}{6} & 0 & \frac{1}{2} - \frac{y}{2} \end{pmatrix}$$

$$N6_1(x,y) := N6(x,y)_{1,1} \quad N6_2(x,y) := N6(x,y)_{1,3} \quad N6_3(x,y) := N6(x,y)_{1,5}$$

Formulate B matrix for each element:

Elements 1,3,5:

$$\text{Bodd}(x, y) := \begin{bmatrix} \left(\frac{d}{dx}N1_1(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N1_1(x, y)) & \left(\frac{d}{dx}N1_2(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N1_2(x, y)) & \left(\frac{d}{dx}N1_3(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N1_3(x, y)) \\ (0 \cdot N1_1(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N1_1(x, y)\right) & (0 \cdot N1_2(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N1_2(x, y)\right) & (0 \cdot N1_3(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N1_3(x, y)\right) \\ \left(\frac{d}{dy}N1_1(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N1_1(x, y)\right) & \left(\frac{d}{dy}N1_2(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N1_2(x, y)\right) & \left(\frac{d}{dy}N1_3(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N1_3(x, y)\right) \end{bmatrix}$$

$$\text{Bodd}(x, y) \rightarrow \begin{pmatrix} -\frac{1}{6} & 0 & \frac{1}{6} & 0 & 0 & 0 \\ 0 & -\frac{1}{2} & 0 & 0 & 0 & \frac{1}{2} \\ -\frac{1}{2} & -\frac{1}{6} & 0 & \frac{1}{6} & \frac{1}{2} & 0 \end{pmatrix} \quad \underline{\underline{\text{Bodd}}} := \begin{pmatrix} -\frac{1}{6} & 0 & \frac{1}{6} & 0 & 0 & 0 \\ 0 & -\frac{1}{2} & 0 & 0 & 0 & \frac{1}{2} \\ -\frac{1}{2} & -\frac{1}{6} & 0 & \frac{1}{6} & \frac{1}{2} & 0 \end{pmatrix}$$

Elements 2,4,6:

$$\text{Beven}(x, y) := \begin{bmatrix} \left(\frac{d}{dx}N2_1(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N2_1(x, y)) & \left(\frac{d}{dx}N2_2(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N2_2(x, y)) & \left(\frac{d}{dx}N2_3(x, y)\right) + (0 \cdot 0) & \left(\frac{d}{dx}0\right) + (0 \cdot N2_3(x, y)) \\ (0 \cdot N2_1(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N2_1(x, y)\right) & (0 \cdot N2_2(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N2_2(x, y)\right) & (0 \cdot N2_3(x, y)) + \left(\frac{d}{dy}0\right) & (0 \cdot 0) + \left(\frac{d}{dy}N2_3(x, y)\right) \\ \left(\frac{d}{dy}N2_1(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N2_1(x, y)\right) & \left(\frac{d}{dy}N2_2(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N2_2(x, y)\right) & \left(\frac{d}{dy}N2_3(x, y)\right) + \left(\frac{d}{dx}0\right) & \left(\frac{d}{dy}0\right) + \left(\frac{d}{dx}N2_3(x, y)\right) \end{bmatrix}$$

$$\text{Beven}(x, y) \rightarrow \begin{pmatrix} \frac{1}{6} & 0 & -\frac{1}{6} & 0 & 0 & 0 \\ 0 & \frac{1}{2} & 0 & 0 & 0 & -\frac{1}{2} \\ \frac{1}{2} & \frac{1}{6} & 0 & -\frac{1}{6} & -\frac{1}{2} & 0 \end{pmatrix} \quad \underline{\underline{\text{Beven}}} := \begin{pmatrix} \frac{1}{6} & 0 & -\frac{1}{6} & 0 & 0 & 0 \\ 0 & \frac{1}{2} & 0 & 0 & 0 & -\frac{1}{2} \\ \frac{1}{2} & \frac{1}{6} & 0 & -\frac{1}{6} & -\frac{1}{2} & 0 \end{pmatrix}$$

Find the plane stress elasticity matrix:

$$E := 27000 \cdot 10^3 \quad \text{lb/in}^2$$

$$\nu := 0.25$$

$$t := 0.25 \quad \text{in}$$

$$A_{\text{elem}} := 0.5 \cdot 6 \cdot 2 = 6 \quad \text{in}^2$$

For plane stress, $\sigma_z = 0$.

The elasticity matrix depends on Hooke's Law:

$$\sigma_{xx} = (E/1-\nu^2) \cdot (\epsilon_{xx} + \nu\epsilon_{yy})$$

$$\sigma_{yy} = (E/1-\nu^2) \cdot (\nu\epsilon_{xx} + \epsilon_{yy})$$

$$\sigma_{xy} = (E/1-\nu^2) \cdot (1-\nu)\epsilon_{xy}$$

Therefore:

$$C := \left(\frac{E}{1-\nu^2} \right) \cdot \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{(1-\nu)}{2} \end{bmatrix}$$

$$C = \begin{pmatrix} 2.88 \times 10^7 & 7.2 \times 10^6 & 0 \\ 7.2 \times 10^6 & 2.88 \times 10^7 & 0 \\ 0 & 0 & 1.08 \times 10^7 \end{pmatrix} \quad \text{lb/in}^2$$

Find element stiffness matrices:

Odd Elements:

$$k_{\text{odd}} := (t \cdot A_{\text{elem}}) \cdot (\text{Bodd}^T \cdot C \cdot \text{Bodd})$$

$$k_{\text{odd}} = \begin{pmatrix} 5.25 \times 10^6 & 2.25 \times 10^6 & -1.2 \times 10^6 & -1.35 \times 10^6 & -4.05 \times 10^6 & -9 \times 10^5 \\ 2.25 \times 10^6 & 1.13 \times 10^7 & -9 \times 10^5 & -4.5 \times 10^5 & -1.35 \times 10^6 & -1.08 \times 10^7 \\ -1.2 \times 10^6 & -9 \times 10^5 & 1.2 \times 10^6 & 0 & 0 & 9 \times 10^5 \\ -1.35 \times 10^6 & -4.5 \times 10^5 & 0 & 4.5 \times 10^5 & 1.35 \times 10^6 & 0 \\ -4.05 \times 10^6 & -1.35 \times 10^6 & 0 & 1.35 \times 10^6 & 4.05 \times 10^6 & 0 \\ -9 \times 10^5 & -1.08 \times 10^7 & 9 \times 10^5 & 0 & 0 & 1.08 \times 10^7 \end{pmatrix} \quad \text{lb/in}$$

Because $B_{odd} = -B_{even}$, and $[k] = [B]^T [C] [B] t A_{elem}$

$$[k_{odd}] = [B_{odd}]^T [C] [B_{odd}] t A_{elem}$$

$$[k_{even}] = [B_{even}]^T [C] [B_{even}] t A_{elem} = -[B_{odd}]^T [C] [-B_{odd}] t A_{elem}$$

the negatives cancel and $[k_{odd}] = [k_{even}]$, thus all elements have the same stiffness matrix.

$$\text{Stiff} := k_{odd}$$

Find element force vectors:

Body Forces:

$$B_x := 0 \quad B_y := -3 \quad \text{lb/in}^3$$

$$\text{Body} := \begin{pmatrix} B_x \\ B_y \end{pmatrix}$$

Generalized body force vector Q_{bar} :

Where the diagonal of element 1 is defined by the line $y = -(1/3)x + 1$:

$$Q1_{bar}(x, y) := t \cdot \int_0^6 \int_0^{1-\frac{x}{3}} \left(N_{bar}_i(x, y)^T \right) \cdot \text{Body} \, dy \, dx$$

$$Q1(x, y) := \left(A1^{-1} \right)^T \cdot Q1_{bar}(x, y)$$

$$Q1(x, y) \rightarrow \begin{pmatrix} 0 \\ -0.375 \\ 0 \\ 0.75 \\ 0 \\ -0.375 \end{pmatrix} \quad \text{lb}$$

Where the diagonal of element 2 is defined by the line $y = -(1/3)x + 1$:

$$Q2_{\text{bar}}(x, y) := t \cdot \int_0^6 \int_0^{1-\frac{x}{3}} \left(N_{\text{bar}_1}(x, y)^T \right) \cdot \text{Body} \, dy \, dx$$

$$Q2(x, y) := \left(A2^{-1} \right)^T \cdot Q2_{\text{bar}}(x, y)$$

$$Q2(x, y) \rightarrow \begin{pmatrix} 0 \\ 0.375 \\ 0 \\ -0.75 \\ 0 \\ 0.375 \end{pmatrix} \quad \text{lb}$$

Where the diagonal of element 3 is defined by the line $y = -(1/3)x + 3$:

$$Q3_{\text{bar}}(x, y) := t \cdot \int_6^{12} \int_0^{3-\frac{x}{3}} \left(N_{\text{bar}_1}(x, y)^T \right) \cdot \text{Body} \, dy \, dx$$

$$Q3(x, y) := \left(A3^{-1} \right)^T \cdot Q3_{\text{bar}}(x, y)$$

$$Q3(x, y) \rightarrow \begin{pmatrix} 0 \\ -0.375 \\ 0 \\ 0.75 \\ 0 \\ -0.375 \end{pmatrix} \quad \text{lb}$$

Where the diagonal of element 4 is defined by the line $y = -(1/3)x + 3$:

$$Q4_{\text{bar}}(x, y) := t \cdot \int_6^{12} \int_0^{3-\frac{x}{3}} \left(N_{\text{bar}_1}(x, y)^T \right) \cdot \text{Body} \, dy \, dx$$

$$Q4(x, y) := (A4^{-1})^T \cdot Q4bar(x, y)$$

$$Q4(x, y) \rightarrow \begin{pmatrix} 0 \\ 0.375 \\ 0 \\ -0.75 \\ 0 \\ 0.375 \end{pmatrix} \quad \text{lb}$$

Where the diagonal of element 5 is defined by the line $y = -(1/3)x + 5$:

$$Q5bar(x, y) := t \cdot \int_{12}^{18} \int_0^{5-\frac{x}{3}} (Nbar_i(x, y)^T) \cdot \text{Body} \, dy \, dx$$

$$Q5(x, y) := (A5^{-1})^T \cdot Q5bar(x, y)$$

$$Q5(x, y) \rightarrow \begin{pmatrix} 0 \\ -0.375 \\ 0 \\ 0.75 \\ 0 \\ -0.375 \end{pmatrix} \quad \text{lb}$$

Where the diagonal of element 6 is defined by the line $y = -(1/3)x + 5$:

$$Q6bar(x, y) := t \cdot \int_{12}^{18} \int_0^{5-\frac{x}{3}} (Nbar_i(x, y)^T) \cdot \text{Body} \, dy \, dx$$

$$Q6(x, y) := (A6^{-1})^T \cdot Q6bar(x, y)$$

$$Q6(x, y) \rightarrow \begin{pmatrix} 0 \\ 0.375 \\ 0 \\ -0.75 \\ 0 \\ 0.375 \end{pmatrix} \quad \text{lb}$$

$$Q_{e_{\text{odd}}} := \begin{pmatrix} 0 \\ -0.375 \\ 0 \\ 0.75 \\ 0 \\ -0.375 \end{pmatrix} \text{ lb} \quad Q_{e_{\text{even}}} := \begin{pmatrix} 0 \\ 0.375 \\ 0 \\ -0.75 \\ 0 \\ 0.375 \end{pmatrix} \text{ lb}$$

Surface Traction:

Only on element 6.

$$\text{Surface} := \begin{pmatrix} 0 \\ -2400 \end{pmatrix} \text{ lb/in}^2$$

$$P_6(x, y) := t \cdot \int_{-1}^1 N_6(18, y)^T \cdot \text{Surface} \, dy$$

$$P_6(x, y) \rightarrow \begin{pmatrix} 0 \\ -600.0 \\ 0 \\ 0 \\ 0 \\ -600.0 \end{pmatrix} \text{ lb} \quad P_{e_6} := \begin{pmatrix} 0 \\ -600.0 \\ 0 \\ 0 \\ 0 \\ -600.0 \end{pmatrix} \text{ lb}$$

Combine force vectors for each element:

$$F_{e_1} := Q_{e_{\text{odd}}}$$

$$F_{e_2} := Q_{e_{\text{even}}}$$

$$F_{e_3} := Q_{e_{\text{odd}}} \quad F_{e_5} := Q_{e_{\text{odd}}}$$

$$F_{e_4} := Q_{e_{\text{even}}} \quad F_{e_6} := Q_{e_{\text{even}}} + P_{e_6}$$

Assembly of Global Stiffness Matrix:

$n_elements := 6$ $n_nodes := 8$ $n_free := 2$ $n_dof := n_nodes \cdot n_free$ $nod_el := 3$

$$elem := \begin{pmatrix} 2 & 4 & 1 \\ 3 & 1 & 4 \\ 4 & 6 & 3 \\ 5 & 3 & 6 \\ 6 & 8 & 5 \\ 7 & 5 & 8 \end{pmatrix}$$

$ie := 1..n_elements$ $j := 1..nod_el$ $k := 0..n_free - 1$

$m := 1..nod_el \cdot n_free$ $n := 1..nod_el \cdot n_free$

$top_{ie, n_free \cdot j - k} := n_free \cdot elem_{ie, j} - k$

$$top = \begin{pmatrix} 3 & 4 & 7 & 8 & 1 & 2 \\ 5 & 6 & 1 & 2 & 7 & 8 \\ 7 & 8 & 11 & 12 & 5 & 6 \\ 9 & 10 & 5 & 6 & 11 & 12 \\ 11 & 12 & 15 & 16 & 9 & 10 \\ 13 & 14 & 9 & 10 & 15 & 16 \end{pmatrix}$$

$K_{n_dof, n_dof} := 0$

$K_{(top_{ie, m}, top_{ie, n})} := K_{(top_{ie, m}, top_{ie, n})} + Stiff_{m, n}$

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
1	5.25	0	-4.05	-1.35	-1.2	-0.9	0	2.25	0	0	0	0	0	0	0	0
2	0	11.25	-0.9	-10.8	-1.35	-0.45	2.25	0	0	0	0	0	0	0	0	0
3	-4.05	-0.9	5.25	2.25	0	0	-1.2	-1.35	0	0	0	0	0	0	0	0
4	-1.35	-10.8	2.25	11.25	0	0	-0.9	-0.45	0	0	0	0	0	0	0	0
5	-1.2	-1.35	0	0	10.5	2.25	-8.1	-2.25	-1.2	-0.9	0	2.25	0	0	0	0
6	-0.9	-0.45	0	0	2.25	22.5	-2.25	-21.6	-1.35	-0.45	2.25	0	0	0	0	0
7	0	2.25	-1.2	-0.9	-8.1	-2.25	10.5	2.25	0	0	-1.2	-1.35	0	0	0	0
8	2.25	0	-1.35	-0.45	-2.25	-21.6	2.25	22.5	0	0	-0.9	-0.45	0	0	0	0
9	0	0	0	0	-1.2	-1.35	0	0	10.5	2.25	-8.1	-2.25	-1.2	-0.9	0	2.25
10	0	0	0	0	-0.9	-0.45	0	0	2.25	22.5	-2.25	-21.6	-1.35	-0.45	2.25	0
11	0	0	0	0	0	2.25	-1.2	-0.9	-8.1	-2.25	10.5	2.25	0	0	-1.2	-1.35
12	0	0	0	0	2.25	0	-1.35	-0.45	-2.25	-21.6	2.25	22.5	0	0	-0.9	-0.45
13	0	0	0	0	0	0	0	0	-1.2	-1.35	0	0	5.25	2.25	-4.05	-0.9
14	0	0	0	0	0	0	0	0	-0.9	-0.45	0	0	2.25	11.25	-1.35	-10.8
15	0	0	0	0	0	0	0	0	0	2.25	-1.2	-0.9	-4.05	-1.35	5.25	0
16	0	0	0	0	0	0	0	0	2.25	0	-1.35	-0.45	-0.9	-10.8	0	11.25

K =

$\cdot 10^6$

Unit =lb/in

Assembly of Global Force Matrix:

$$F_{m_dof,1} := 0$$

$$F_{(topic,m,1)} := F_{(topic,m,1)} + (F_{e_ic})_{m,1}$$

F =

	1
1	0
2	-1.13
3	0
4	-0.38
5	0
6	-0.75
7	0
8	0.75
9	0
10	-0.75
11	0
12	0.75
13	0
14	-599.63
15	0
16	-598.88

lb

Apply Boundary Conditions:

$$F_{red} := \text{submatrix}(F, 5, 16, 1, 1)$$

$$F_{red} =$$

	1
1	0
2	-0.75
3	0
4	0.75
5	0
6	-0.75
7	0
8	0.75
9	0
10	-599.63
11	0
12	-598.88

lb

$$K_{\text{red}} := \text{submatrix}(K, 5, 16, 5, 16)$$

$$K_{\text{red}} = \begin{array}{c|cccccccccccc} & 1 & 2 & 3 & 4 & 5 & 6 & 7 & 8 & 9 & 10 & 11 & 12 \\ \hline 1 & 10.5 & 2.25 & -8.1 & -2.25 & -1.2 & -0.9 & 0 & 2.25 & 0 & 0 & 0 & 0 \\ 2 & 2.25 & 22.5 & -2.25 & -21.6 & -1.35 & -0.45 & 2.25 & 0 & 0 & 0 & 0 & 0 \\ 3 & -8.1 & -2.25 & 10.5 & 2.25 & 0 & 0 & -1.2 & -1.35 & 0 & 0 & 0 & 0 \\ 4 & -2.25 & -21.6 & 2.25 & 22.5 & 0 & 0 & -0.9 & -0.45 & 0 & 0 & 0 & 0 \\ 5 & -1.2 & -1.35 & 0 & 0 & 10.5 & 2.25 & -8.1 & -2.25 & -1.2 & -0.9 & 0 & 2.25 \\ 6 & -0.9 & -0.45 & 0 & 0 & 2.25 & 22.5 & -2.25 & -21.6 & -1.35 & -0.45 & 2.25 & 0 \\ 7 & 0 & 2.25 & -1.2 & -0.9 & -8.1 & -2.25 & 10.5 & 2.25 & 0 & 0 & -1.2 & -1.35 \\ 8 & 2.25 & 0 & -1.35 & -0.45 & -2.25 & -21.6 & 2.25 & 22.5 & 0 & 0 & -0.9 & -0.45 \\ 9 & 0 & 0 & 0 & 0 & -1.2 & -1.35 & 0 & 0 & 5.25 & 2.25 & -4.05 & -0.9 \\ 10 & 0 & 0 & 0 & 0 & -0.9 & -0.45 & 0 & 0 & 2.25 & 11.25 & -1.35 & -10.8 \\ 11 & 0 & 0 & 0 & 0 & 0 & 2.25 & -1.2 & -0.9 & -4.05 & -1.35 & 5.25 & 0 \\ 12 & 0 & 0 & 0 & 0 & 2.25 & 0 & -1.35 & -0.45 & -0.9 & -10.8 & 0 & 11.25 \end{array} \cdot 10^6 \text{ lb/in}$$

$$D := K_{\text{red}}^{-1} \cdot F_{\text{red}}$$

$$D = \begin{array}{c|c} & 1 \\ \hline 1 & 1.76 \cdot 10^{-3} \\ 2 & -6.55 \cdot 10^{-3} \\ 3 & -1.7 \cdot 10^{-3} \\ 4 & -6.48 \cdot 10^{-3} \\ 5 & 2.84 \cdot 10^{-3} \\ 6 & -0.02 \\ 7 & -2.67 \cdot 10^{-3} \\ 8 & -0.02 \\ 9 & 3.24 \cdot 10^{-3} \\ 10 & -0.04 \\ 11 & -2.96 \cdot 10^{-3} \\ 12 & -0.04 \end{array} \text{ in}$$

Displacement at free end = -0.040 inches.