## CE 371.01 - Structural Analysis I

Homework \#7
24. Using the moment-area theorems, determine the slope at point $B$ and the deflection at point $C$. $E$ and $I$ are constant over the length of the member. Assume that the support at $A$ is a roller and the support at $B$ is a pin.


Sol:



By symmetry of loading, $\mathrm{A}_{\mathrm{Y}}=\mathrm{B}_{\mathrm{Y}}=10 \mathrm{k}$
From the M/EI diagram,

$$
\mathrm{A}_{1}=\mathrm{A}_{2}=\mathrm{A}_{3}=\mathrm{A}_{4}=-250 / \mathrm{EI}
$$

$\theta_{\mathrm{B}}: \quad \theta_{\mathrm{B}}\left(20^{\prime}\right)=\mathrm{t}_{\mathrm{A} / \mathrm{B}}=\mathrm{A}_{2}(10 / 3)+\mathrm{A}_{3}[10+2(10) / 3]$

$$
\begin{aligned}
\theta_{\mathrm{B}} & =\left[-250\left(10^{\prime}\right) / \mathrm{EI}^{*} 3-250\left(10^{\prime}+20^{\prime} / 3\right) / \mathrm{EI}\right] / 20^{\prime} \\
& =250 / \mathrm{EI} \quad \mathbb{\Omega}>
\end{aligned}
$$

$\mathrm{t}_{\mathrm{A} / \mathrm{B}}: \quad=\theta_{\mathrm{B}}\left(20^{\prime}\right)=250(20) / \mathrm{EI}=5000 / \mathrm{EI} \uparrow$
$\mathrm{t}_{\mathrm{C} / \mathrm{B}}: \quad \mathrm{t}_{\mathrm{C} / \mathrm{B}} \downarrow=\theta_{\mathrm{B}} * 2(10) / 3=250(20) / 3 * \mathrm{EI}=1667 / \mathrm{EI} \downarrow$

$$
\begin{aligned}
& \Delta^{\prime}(\operatorname{tanB} \longrightarrow \mathrm{pt} \mathrm{C}): \quad=1 / 2\left(\mathrm{t}_{\mathrm{A} / \mathrm{B}}\right)=2500 / \mathrm{EI} \uparrow \\
& \Delta_{\mathrm{C}}=\Delta^{\prime} \uparrow-\mathrm{t}_{\mathrm{C} / \mathrm{B}} \downarrow=2500 / \mathrm{EI}-1667 / \mathrm{EI}=833 / \mathrm{EI} \uparrow
\end{aligned}
$$

Summary:

$$
\begin{aligned}
& \theta_{\mathrm{B}}=250 / \mathrm{EI} \mathbb{\mathbb { }}> \\
& \Delta_{\mathrm{C}}=833 / \mathrm{EI} \uparrow
\end{aligned}
$$

Ans (Points 5)
Ans (Points 5)
25. Using the moment-area theorems, determine the slope just to the left and just to the right of the hinge at $B$. Also determine the deflection at $D$. Assume the beam is fixed at $A$ and that $C$ is a roller. $E$ and $I$ are constant over the length of the structure.


Sol:


Cut @ hinge: BCD

$$
\mathbb{N} \Sigma \mathrm{M}_{\mathrm{B}}=0=5 \mathrm{kN} \cdot \mathrm{~m}-\mathrm{C}_{\mathrm{Y}}(5 \mathrm{~m})
$$

$$
\mathrm{C}_{\mathrm{Y}}=1 \mathrm{kN} \uparrow
$$

Full Structure:

$$
\begin{aligned}
& +\Sigma \mathrm{F}_{\mathrm{Y}}=0=-\mathrm{A}_{\mathrm{Y}}+1 \\
& \mathrm{~A}_{\mathrm{Y}}=1 \mathrm{kN} \downarrow \\
& \text { (1) } \Sigma \mathrm{M}_{\mathrm{A}}=0=-\mathrm{M}_{\mathrm{A}}-5 \mathrm{kN} \cdot \mathrm{~m}+1 \mathrm{kN}(10 \mathrm{~m}) \\
& \mathrm{M}_{\mathrm{A}}=5 \mathrm{kN} \cdot \mathrm{~m} \mathbb{A}
\end{aligned}
$$

$\mathrm{A}_{1}=5^{2} / 2 * \mathrm{EI}=12.5 / \mathrm{EI}$
$\mathrm{A}_{2}=12.5 / \mathrm{EI}$
$\mathrm{A}_{3}=25 / \mathrm{EI}$
$\theta_{\mathrm{BL}}: \quad \theta_{\mathrm{BL}}=\theta_{\mathrm{B} / \mathrm{A}} \hat{\boldsymbol{\omega}}=\mathrm{A}_{1}=12.5 / \mathrm{EI}<\hat{\mathbb{J}}$
$\theta_{\mathrm{C}}: \quad \theta_{\mathrm{C}}=\left(\Delta_{\mathrm{B}}+\mathrm{t}_{\mathrm{B} / \mathrm{C}}\right) / 5 \mathrm{~m}=\left[\mathrm{A}_{1}(2 * 5 / 3)+\mathrm{A}_{2}(2 * 5 / 3)\right] / 5 \mathrm{~m}$

$$
\begin{aligned}
& \theta_{\mathrm{C}}=12.5(2) /(3 * \mathrm{EI})+12.5(2) /(3 * \mathrm{EI})=16.67 \mathrm{EI} \curvearrowright \\
\theta_{\mathrm{BR}}: & \theta_{\mathrm{BR}}=\theta_{\mathrm{C}} \mathbb{A}>-\theta_{\mathrm{B} / \mathrm{C}} \neq>=16.67 / \mathrm{EI}-\mathrm{A} 2=16.67 / \mathrm{EI}-12.5 / \mathrm{EI}=4.17 / \mathrm{EI} \curvearrowright> \\
\Delta_{\mathrm{D}}: & \Delta_{\mathrm{D}}=\theta_{\mathrm{C}} \downarrow(5 \mathrm{~m})+\mathrm{t}_{\mathrm{D} / \mathrm{C}} \downarrow=16.67(5 \mathrm{~m}) / \mathrm{EI}+\mathrm{A}_{3}(5 \mathrm{~m}) / 2 \\
& \Delta_{\mathrm{D}}=16.67(5) /(\mathrm{EI})+25(5) /(2 * \mathrm{EI})=145.85 / \mathrm{EI} \downarrow
\end{aligned}
$$

Summary

$$
\begin{aligned}
& \theta_{\mathrm{L}}=12.5 / \mathrm{EI}<\mathbb{\vartheta} \\
& \theta_{\mathrm{BR}}=4.17 / \mathrm{EI} \mathbb{\wedge}> \\
& \Delta_{\mathrm{D}}=145.85 / \mathrm{EI} \downarrow
\end{aligned}
$$

Ans (Points 4)
Ans (Points 4)
Ans (Points 2)
26. Use the moment-area theorems and determine the value of $a$ so that the slope at $A$ is equal to zero. $E I$ is constant.


Sol:


Moment Area Theorems:

$$
\begin{aligned}
& \theta_{\mathrm{A}}=\left(\theta_{\mathrm{A} / \mathrm{C}}\right)_{1}=1 / 2(\mathrm{PL} / 4 \mathrm{EI})(\mathrm{L} / 2)=\mathrm{PL}^{2} / 16 \mathrm{EI} \\
& \left(\mathrm{t}_{\mathrm{B} / \mathrm{A}}\right)_{2}=1 / 2(-\mathrm{Pa} / \mathrm{EI})(\mathrm{L})(2 \mathrm{~L} / 3)=\mathrm{Pa}^{2} / 3 \mathrm{EI} \\
& \left(\theta_{\mathrm{A}}\right)_{2}=\mid\left(\mathrm{t}_{\mathrm{B} / \mathrm{A}}\right)_{2} / 2=\left(-\mathrm{PaL}{ }^{2} / 3 \mathrm{EI}\right) / \mathrm{L}=\mathrm{PaL} / 3 \mathrm{EI}
\end{aligned}
$$

Require, $\quad \theta_{\mathrm{A}}=0=\left(\theta_{\mathrm{A}}\right)_{1}-\left(\theta_{\mathrm{A}}\right)_{2}$

$$
0=\mathrm{PL}^{2} / 16 \mathrm{EI}-\mathrm{PaL} / 3 \mathrm{EI}
$$

$$
\mathrm{a}=3 \mathrm{~L} / 16
$$

Ans (Points 10)
27. Using the moment-area theorems, determine the horizontal and vertical components of displacement at $C$. Let $E=29,000$ ksi and $I=80$ in $^{4}$ for each member.


Sol:


$$
\not \ddagger^{\Sigma \mathrm{F}_{\mathrm{X}}=0 ;} \quad \mathrm{AX}-0.2(8)=0
$$

$$
\mathrm{A}_{\mathrm{X}}=1.6 \mathrm{k} \rightarrow
$$

$$
\omega \Sigma \mathrm{M}_{\mathrm{A}}=0 ; \quad-1.6(4)+\mathrm{M}_{\mathrm{A}}=0
$$

$$
\mathrm{M}_{\mathrm{A}}=6.4 \mathrm{k} . \mathrm{ft} \mathbb{V}
$$

$$
\mathrm{A}_{1}=64 / \mathrm{EI} \mathrm{kft}^{2}
$$

$$
\mathrm{A}_{2}=6.4(8) / 3 \mathrm{EI}=17.07 / \mathrm{EI}
$$

$\theta_{\mathrm{B}}=\mathrm{A}_{1}=64 / \mathrm{EI}<\boldsymbol{\wedge}$
$\Delta \mathrm{B}_{\mathrm{Y}}=\Delta \mathrm{C}_{\mathrm{Y}}=\mathrm{A}_{1}\left(5^{\prime}\right)=64(5) / \mathrm{EI}=320 / \mathrm{EI} \downarrow$

$$
\begin{aligned}
& \theta_{\mathrm{C} / \mathrm{B}}=\mathrm{A}_{2}=17.07 / \mathrm{EI} \curvearrowleft \\
& \mathrm{t}_{\mathrm{C} / \mathrm{B}}=\mathrm{A}_{2}(3 / 4)(8)=17.07(3)(2) / \mathrm{EI}=102.42 / \mathrm{EI} \\
& \theta_{\mathrm{B}}(8)=64(8) / \mathrm{EI}=512 / \mathrm{EI} \leftarrow \\
& \Delta \mathrm{C}_{\mathrm{X}}=(102.42+512) / \mathrm{EI}=614.42 / \mathrm{EI} \leftarrow \\
& \mathrm{EI}=29000 \mathrm{ksi}\left(80 \mathrm{in}^{4}\right)=2.32 \times 10^{6}{\mathrm{k} . \mathrm{in}^{2}} \\
& \Delta \mathrm{C}_{\mathrm{Y}}=320(1728) / 2.32 \times 10^{6} \mathrm{in}=0.24^{\prime \prime} \downarrow \\
& \Delta \mathrm{C}_{\mathrm{X}}=614.42(1728) / 2.32 \times 10^{6}=0.46^{\prime \prime} \leftarrow
\end{aligned}
$$

Summary:
$\Delta \mathrm{C}_{\mathrm{Y}}=0.24^{\prime} \downarrow$
$\Delta \mathrm{C}_{\mathrm{X}}=0.46 " \leftarrow$

Ans (Points 5)
Ans (Points 5)
28. Using the moment-area theorems, determine the slope at $A$ and the deflection of $B$. The moment of inertia for each member is indicated in the figure, and $E=29,000 \mathrm{ksi}$. Assume $A$ is a roller support and $D$ is a pin support.


Sol:


M/EI


$$
+\Sigma \mathrm{F}_{Y} ; \quad \mathrm{A}_{Y}+\mathrm{D}_{Y}-5(8)
$$

By Symmetry:

$$
\mathrm{A}_{\mathrm{Y}}=\mathrm{D}_{\mathrm{Y}}=5(4)=20 \mathrm{k} \uparrow
$$

M/EI @centre-line: $\quad 5 * 8^{2} / 8 * E I=40 / E I ~ k f t$

$$
\mathrm{A} 1=\mathrm{A} 2=40(4)(2) / 3 * \mathrm{EI}=106.7 / \mathrm{EI}
$$

$\theta_{\mathrm{B}}=\mathrm{A}_{1}=106.7 / \mathrm{EI} \mathbb{A}$
$\theta_{\mathrm{C}}=106.7 / \mathrm{EI}_{\mathrm{BC}}{ }^{\mathbb{C}}>=\theta_{\mathrm{D}}$
$\Delta_{\mathrm{C}}=\theta_{\mathrm{D}}(12)=\Delta_{\mathrm{B}}=106.7(12) / \mathrm{EI}_{\mathrm{BC}}=1280 \mathrm{kft}^{3} / \mathrm{EI}_{\mathrm{BC}} \mathrm{kin}^{2}$
$\Delta_{\mathrm{B}}=1280(1728) /(29000 * 900)=0.085 " \longleftarrow$
$\theta_{\mathrm{A}}=\Delta_{\mathrm{B}} / 12^{\prime}=0.085^{\prime \prime} / 12\left(12^{\prime \prime}\right)=0.59 \times 10^{-3} \mathrm{rad} \AA>$
Summary:
$\Delta_{\mathrm{B}}=0.085^{\prime \prime} \longleftarrow$
$\theta_{\mathrm{A}}=0.59 \times 10^{-3} \mathrm{rad}<\mathbb{A}_{\text {or }} 33.7 \times 10^{-3}$ (degrees)

Ans (Points 5)
Ans (Points 5)

