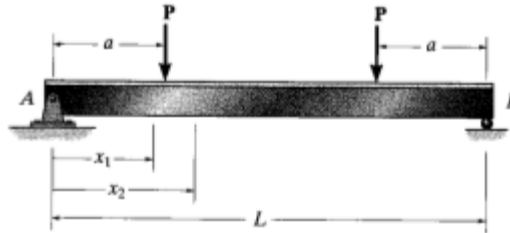


**CE 371 - Section 02**

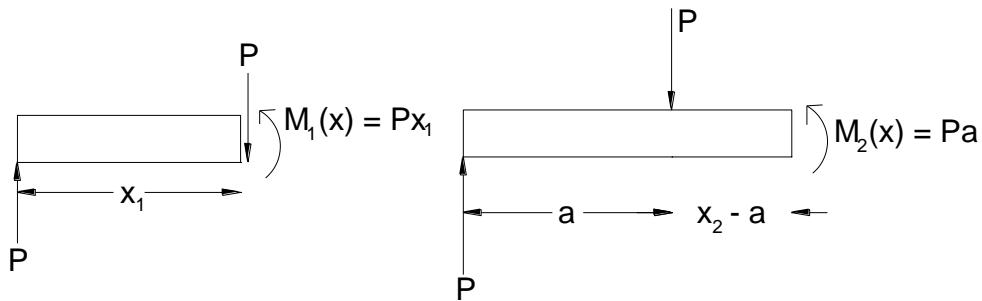
**Homework No. 6 - Solutions**

**8-3**

Determine the equations of the elastic curve for the beam using the  $x_1$  and  $x_2$  coordinates. Specify the slope at A and the maximum deflection. EI is constant



Sol:



$$EI \frac{d^2v}{dx^2} = M(x)$$

For  $M_1(x) = Px_1$

$$EI \frac{d^2v_1}{dx_1^2} = Px_1$$

$$EI \frac{dv_1}{dx_1} = Px_1^2/2 + C_1 \tag{1}$$

$$EIv_1 = Px_1^3/6 + C_1x_1 + C_2 \tag{2}$$

For  $M_2(x) = Pa$

$$EI \frac{d^2v_2}{dx_2^2} = Pa$$

$$EI \frac{dv_2}{dx_2} = Pax_2 + C_3 \tag{3}$$

$$EIv_2 = Pax_2^2/2 + C_3x_2 + C_4 \tag{4}$$

Boundary Conditions

$$v_1 = 0 \text{ at } x = 0$$

From eq. (2)

$$C_2 = 0$$

Due to symmetry:

$$dv_2/dx_2 = 0 \text{ at } x_2 = L/2$$

From eq.(3)

$$0 = PaL/2 + C_3$$

$$C_3 = -PaL/2$$

Continuity conditions:

$$v_1 = v_2 \text{ at } x_1 = x_2 = a$$

$$Pa^3/6 + C_1a = Pa^3/2 - Pa^2L/2 + C_4$$

$$C_1a - C_4 = Pa^3/3 - Pa^2L/2 \quad (5)$$

$$dv_1/dx_1 = dv_2/dx_2 \text{ at } x_1 = x_2 = a$$

$$Pa^2/2 + C_1 = Pa^2 - PaL/2$$

$$C_1 = Pa^2/2 - PaL/2$$

Substitute  $C_1$  into eq. (5)

$$C_4 = Pa^3/6$$

$$dv_1/dx_1 = P(x_1^2 + a^2 - aL) / 2EI$$

$$\theta_A = dv_1/dx_1 \Big|_{x_1=0} = Pa(a - L)/2EI \quad \text{Ans (Points 3)}$$

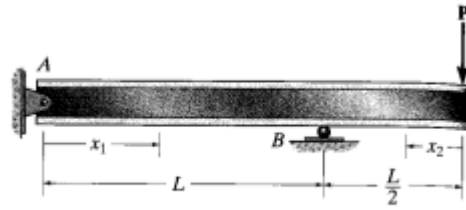
$$v_1 = Px_1\{(x_1^2 + 3a(a - L))\}/6EI \quad \text{Ans (Points 3)}$$

$$v_2 = Pa\{(3x(x - L) + a^2)\}/6EI \quad \text{Ans (Points 3)}$$

$$v_{\max} = v_2 \Big|_{x=L/2} = Pa(4a^2 - 3L^2)/24EI \quad \text{Ans (Point 1)}$$

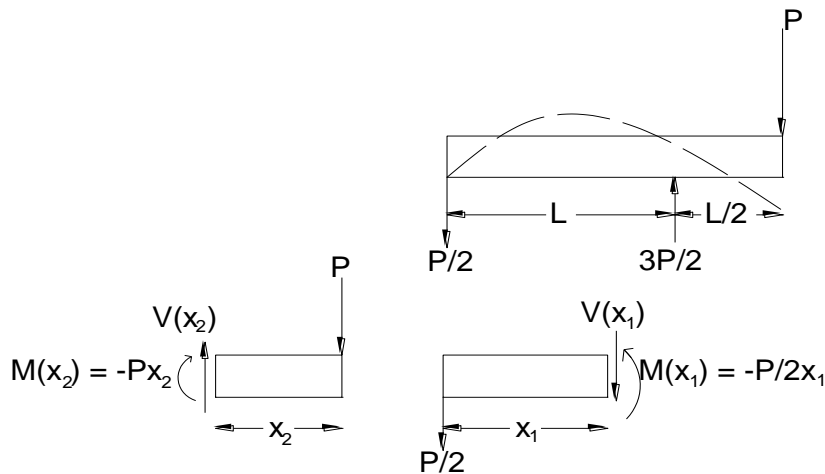
### 8-4

Determine the equations of the elastic curve for the beam using the  $x_1$  and  $x_2$  coordinates. Specify beam's maximum deflection.  $EI$  is constant



Prob. 8-4

Sol:



### Slope and Elastic Curve

$$EI d^2v/dx^2 = M(x)$$

$$\text{For } M(x_1) = -Px_1/2,$$

$$EI d^2v_1/dx_1^2 = -Px_1/2$$

$$EI dv_1/dx_1 = -Px_1^2/4 + C_1 \quad (1)$$

$$EI v_1 = -Px_1^3/12 + C_1x_1 + C_2 \quad (2)$$

$$\text{For } M(x_2) = -Px_2,$$

$$EI d^2v_2/dx_2^2 = -Px_2$$

$$EI dv_2/dx_2 = -Px_2^2/2 + C_3 \quad (3)$$

$$EI v_2 = -Px_2^3/6 = C_3x_2 + C_4 \quad (4)$$

### Boundary Conditions:

$$v_1 = 0 \text{ at } x_1 = 0$$

From eq.(2),  $C_2 = 0$

$v_1 = 0$  at  $x_1 = L$

From eq. (2)

$$0 = -PL^3/12 + C_1L \quad C_1 = PL^2/12$$

$v_2 = 0$  at  $x_2 = L/2$

From eq. (4),

$$0 = -PL^3/48 + LC_3/2 + C_4 \quad (5)$$

### Continuity Conditions:

At  $x_1 = L$  and  $x_2 = L/2$ ,  $dv_1/dx_1 = -dv_2/dx_2$

From eqs (1) and (3),

$$-PL^2/4 + PL^2/12 = -(PL^2/8 + C_3)$$

$$C_3 = 7PL^2/24$$

From eq. (5)

$$C_4 = -PL^3/8$$

### The Slope:

Substitute the value of  $C_1$  into eq. (1)

$$dv_1/dx_1 = P(L^2 - 3x_1^2)/12EI$$

$$dv_1/dx_1 = 0 = P(L^2 - 3x_1^2)/12EI$$

$$x_1 = L/\sqrt{3}$$

### The Elastic Curve:

Substitute the values of  $C_1$ ,  $C_2$ ,  $C_3$  and  $C_4$  into eqs (2) and (4), respectively,

$$v_1 = Px_1(-x_1^2 + L^2)/12EI \quad \text{Ans (Points 4)}$$

$$v_D = v_1 \Big|_{x_1=L/\sqrt{3}} = P(L/\sqrt{3})(-L^2/3 + L^2)/12EI = 0.0321 PL^3/EI$$

$$v_2 = P(-4x_2^3 + 7L^2x_2 - 3L^3)/24EI \quad \text{Ans (Points 4)}$$

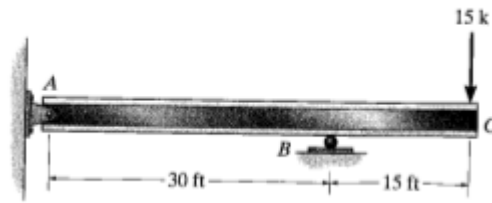
$$v_c = v_2 \Big|_{x_2=0} = -PL^3/8EI$$

Hence,

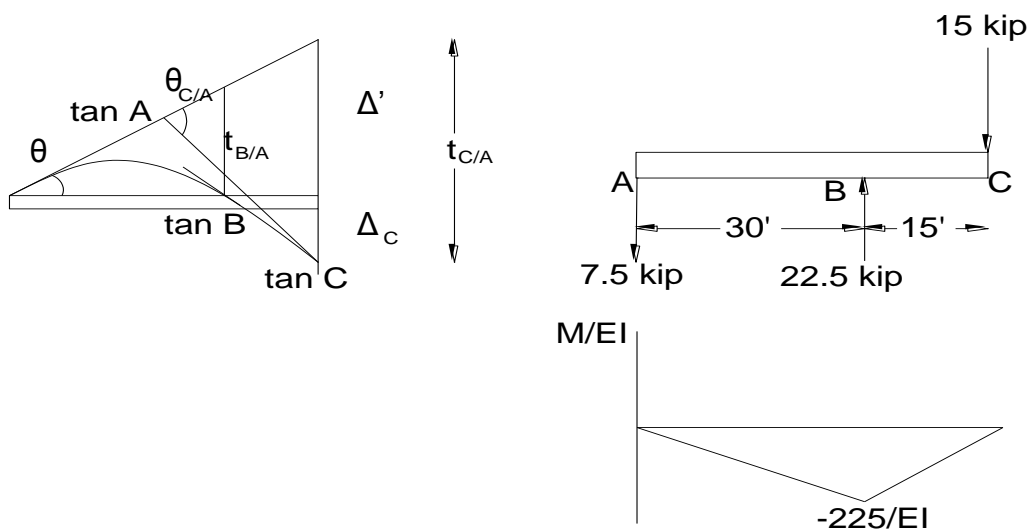
$$v_{\max} = v_c = PL^3/8EI \quad \text{Ans (Points 2)}$$

### 8-12

Use the moment-area theorems to determine the slope and deflection at C. EI is constant



Sol:



$$\theta_A = |t_{B/A}|/30$$

$$t_{B/A} = \frac{1}{2} (-225/EI)(30)(10) = -33750/EI$$

$$\theta_A = 1125/EI$$

$$\theta_{C/A} = \frac{1}{2} (-225/EI)(30) + \frac{1}{2} (-225/EI)(15) = -5062.5/EI = 5062.5/EI$$

$$\theta_C = \theta_{C/A} + \theta_A$$

$$\theta_C = 5062.5/EI - 1125/EI = 3937.5/EI$$

**Ans (Points 5)**

$$\Delta_C = |t_{C/A}| - 45/30 |t_{B/A}|$$

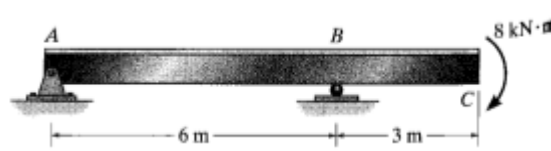
$$t_{C/A} = \frac{1}{2} (-225/EI)(30)(25) + \frac{1}{2} (-225/EI)(15)(10) = -101250/EI$$

$$\Delta_C = 101250/EI - 45/30(33750/EI) = 50625/EI$$

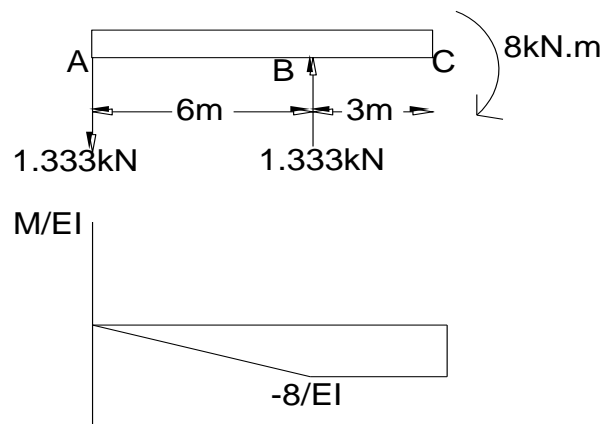
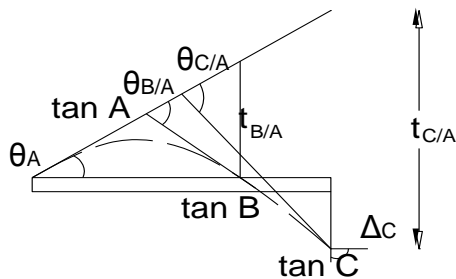
**Ans (Points 5)**

### 8-20

Use the moment-area theorems and determine the deflection at C and the slope of the beam at A, B and C. EI is constant.



Sol:



$$t_{B/A} = \frac{1}{2} (-8/EI)(6)(2) = -48/EI$$

$$t_{C/A} = \frac{1}{2} (-8/EI)(6)(3+2) + (-8/EI)(3)(1.5) = -156/EI$$

$$\Delta_C = |t_{C/A}| - 9/6 |t_{B/A}| = 156/EI - 9(48)/6EI = 84/EI$$

Ans (Points 3)

$$\theta_A = |t_{B/A}| / 6 = 8/EI$$

Ans (Points 3)

$$\theta_{B/A} = \frac{1}{2} (-8/EI)(6) = -24/EI = 24/EI$$

$$\theta_B = \theta_{B/A} + \theta_A$$

$$\theta_B = 24/EI - 8/EI = 16/EI$$

Ans (Points 2)

$$\theta_{C/A} = \frac{1}{2} (-8/EI)(6) + (-8/EI)(3) = -48/EI = 48/EI$$

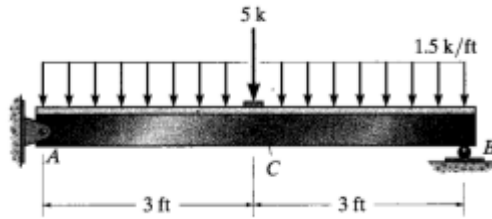
$$\theta_C = \theta_{C/A} + \theta_A$$

$$\theta_C = 48/EI - 8/EI = 40/EI$$

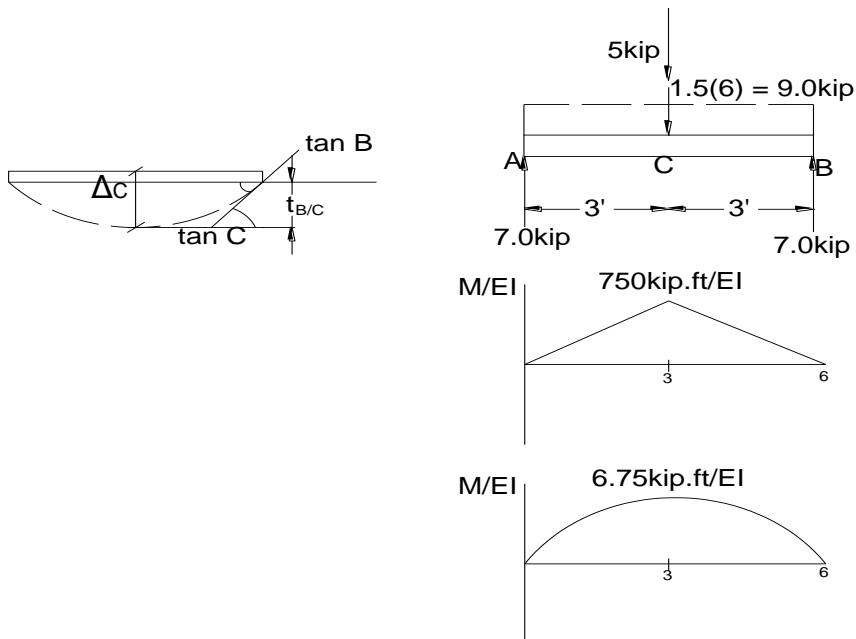
Ans (Points 2)

### 8-24

Use the moment-area theorems and determine the slope at B and the displacement at C. The member is an A-36 steel structure Tee for which  $I = 76.8 \text{ in}^4$ .



Sol:



#### Moment Area Theorems:

Due to symmetry, the slope at midspan C is zero. Hence the slope at B is

$$\begin{aligned}
 \theta_B = \left| \theta_{B/C} \right| &= \frac{1}{2} (7.50/EI) (3) + \frac{2}{3} (6.75/EI) (3) \\
 &= 24.75 \text{ kip.ft}^2/EI \\
 &= 24.75(144) / \{29(10^3) (76.8)\} \\
 &= 0.00160 \text{ rad}
 \end{aligned}$$

**Ans (Points 5)**



The displacement at C is

$$\begin{aligned}\Delta_C = |t_{AC}| &= \frac{1}{2} (7.50/EI)(3)(2/3)(3) + \frac{2}{3}(6.75/EI)(3)(5/8)(3) \\ &= 47.8125 \text{ kip}\cdot\text{ft}^3/EI \\ &= 47.8125(1728) / \{29(10^3)(76.8)\} \\ &= 0.0371 \text{ in} \downarrow\end{aligned}$$

**Ans (Points 5)**